

Declaration of Performance

2873-CPR-M 530-10

1. Unique identification code of the product-type: Mungo injection system MIT-Hybrid Plus for rebar connection

2. Manufacturer: Mungo Befestigungstechnik AG, Bornfeldstrasse 2, CH-4600 Olten - Switzerland

3. System/s of AVCP: System 1

4. Intended use or use/es:

Product	Intended use
Injection system for post installed rebar	Post-installed connection with mortar, by anchoring or overlap joint,
connection with mortar	of reinforcing bars (rebars) in existing structures made of normal
	weight concrete acc. to regulations for reinforced concrete
	constructions

5. European Assessment Document: EAD 330087-00-0601, Edition 05/2018

European Technical Assessment: ETA-17/0130 of 7 June.2019 **Technical Assessment Body:** DIBt – Deutsches Institut für Bautechnik

Notified body/ies: 2873 (IFSW) acc. No. 305/2011 (Construction Product Regulation EU)

6. Declared performance:

Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under static and quasi-static loading	See appendix, especially Annex C1

Safety in case of fire (BWR 2)

Essential characteristic	Performance	
Reaction to fire	Satisfy requirements for Class A1	
Resistance to fire	See Annex C2 and C3	

The performance of the product identified above is in conformity with the set of declared performance/s. This declaration of performance is issued, in accordance with Regulation (EU) No 305/2011, under the sole responsibility of the manufacturer identified above.

Singed for and on behalf of the manufacturer by:

Dipl.-Ing. Robert Klemencic

Head of Engineering

Olten,29.03.2021



This DoP has been prepared in different languages. In case there is a dispute on the interpretation the English version shall always prevail. The Appendix includes voluntary and complementary information in English language exceeding the (language as neutrally specified) legal requirements.



Installation post installed rebar

Figure A1: Overlapping joint for rebar connections of slabs and beams

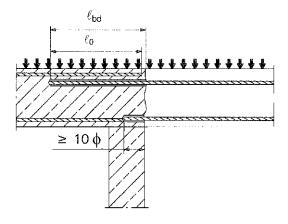


Figure A3: End anchoring of slabs or beams

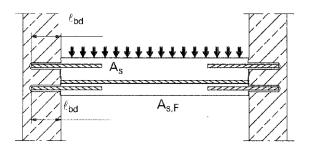


Figure A5: Anchoring of reinforcement to cover the line of acting tensile force

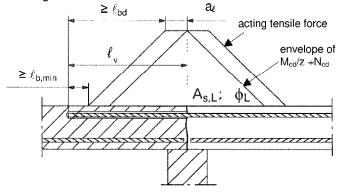


Figure A2: Overlapping joint at a foundation of a wall or column where the rebars are stressed in tension

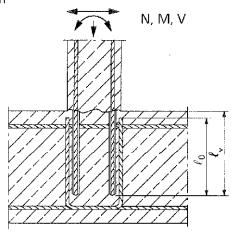
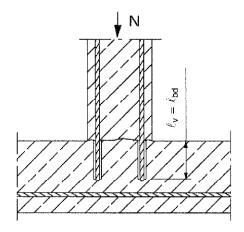


Figure A4: Rebar connection for components stressed primarily in compression. The rebars sre stressed in compression



Note to Figure A1 to A5:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2004+AC:2010.

Preparing of joints according to Annex B 2

Mungo Injection system MIT-Hybrid Plus for rebar connection

Product description

Installed condition and examples of use for rebars

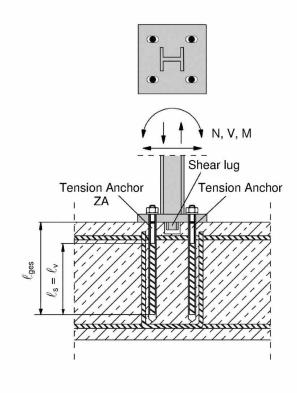
Annex A 1



Installation tension anchor ZA

Figure A6: Overlapping joint of a column stressed in bending to a foundation

Figure A7: Overlap joint for the anchorage of barrier posts



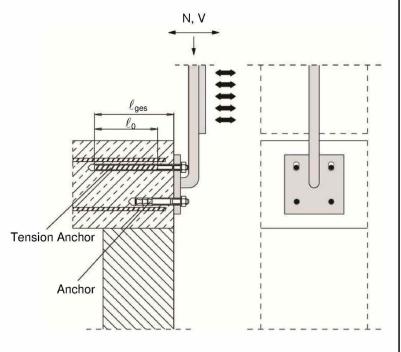
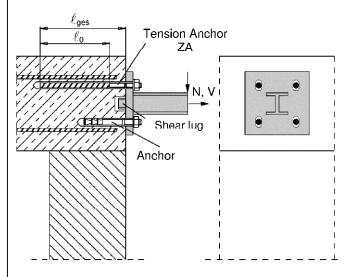


Figure A8: Overlap joint for the anchorage to centilever members



Note to Figure A6 to A8:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2002+AC:2010

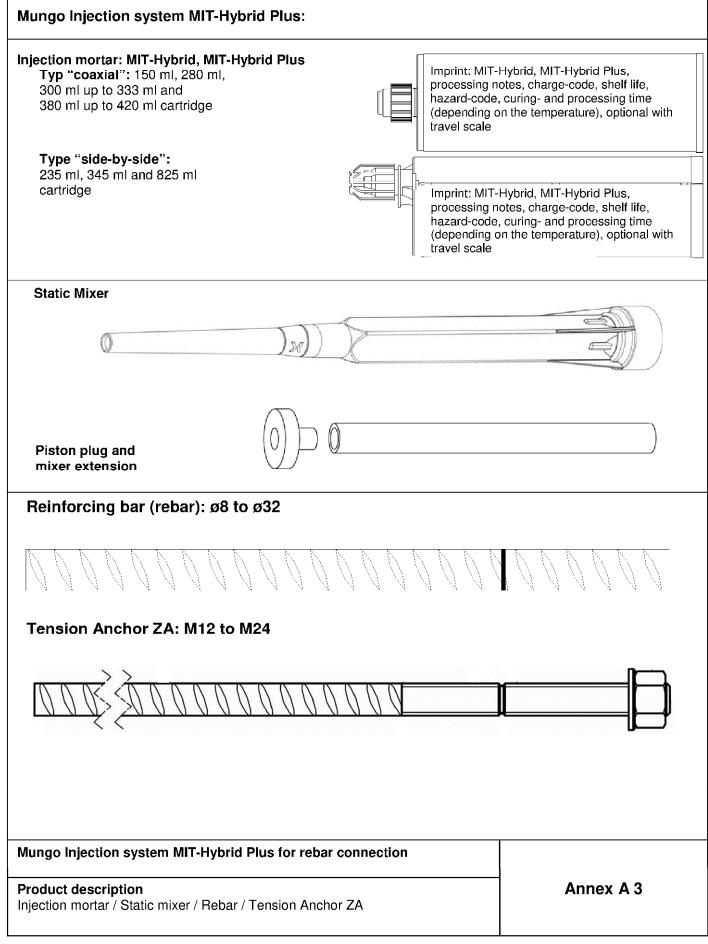
Mungo Injection system MIT-Hybrid Plus for rebar connection

Product description

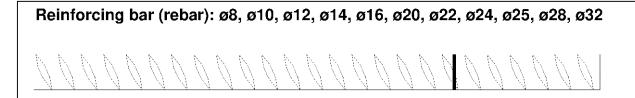
Installed condition and examples of use for tension anchors ZA

Annex A 2









- Minimum value of related rip area f_{R,min} according to EN 1992-1-1:2004+AC:2010
- Rib height of the bar shall be in the range 0,05φ ≤ h ≤ 0,07φ
 (φ: Nominal diameter of the bar; h: Rip height of the bar)

Table A1: Materials

Designation	Material
Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1/NA $f_{uk} = f_{tk} = k \cdot f_{yk}$

Mungo Injection system MIT-Hybrid Plus for rebar connection	
Product description Specifications Rebar	Annex A 4



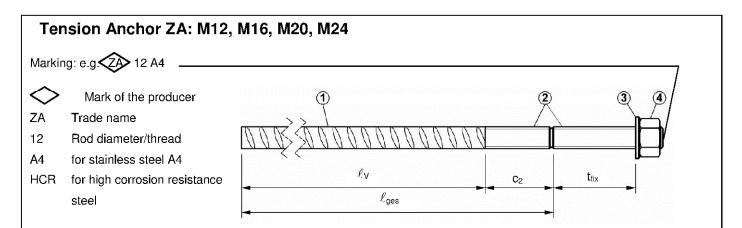


Table A2: Materials

		Material											
Part	Designation	ZA vz			ZA A4			ZA HCR					
	3	M12	M16	M20	M24	M12	M16	M20	M24	M12	M16	M20	M24
1	Reinforcement bar	Class I		ding to l	NDP or	NCL of EN 1992-1-1/NA							
2	Threaded rod	to EN	Steel, zinc plated according to EN 10087:1998 or EN 10263:2001			Stainless steel, 1.4362, 1.4401, 1.4404, 1.4571, EN 10088-1:2014			High corrosion resistant steel, 1.4529, 1.4565, EN 10088-1:2014				
	f _{yk} [N/mm²]	640				640		560		640		560	
3	Washer	Steel, zinc plated according to EN 10087:1998 or			Stainless steel, 1.4362, 1.4401, 1.4404, 1.4571,			High corrosion resistant					
4	Nut	EN 10263:2001			EN 10088-1:2014		steel, 1.4529, 1.4565, EN 10088-1:2014						

Table A3: Dimensions and installation parameter

Size			ZA-M12	ZA-M16	ZA-M20	ZA-M24	
Diameter of threaded rod [mm]		12	16	20	24		
Diameter of reinfo	rcement bar		[mm]	12	16	20	25
Drill hole diameter			[mm]	16	20	25	32
Diameter of cleara	nce hole in fixture		[mm] 14 18 22				26
With across nut fla	its	SW	W [mm] 19 24 30				36
Stress area	A _s [mm²] 84 157 245 35				353		
Effective embedm	ent depth	ℓ_{v}	[mm]		according to st	atic calculation	
Length of bonded	plated		[]	≥ 20	≥ 20	≥ 20	≥ 20
thread	A4/HCR	C ₂	[mm]	≥ 100	≥ 100	≥ 100	≥ 100
Minimum thickness of fixture		t _{fix}	[mm]	5	5	5	5
Maximum thickness of fixture t _{fix} [[mm]	3000	3000	3000	3000	
Maximum installat	ion torque	T _{max} [Nm] 50 100 150				150	

Mungo Injection system MIT-Hybrid Plus for rebar connection	
Product description	Annex A 5
Specifications Tension Anchor ZA	



Specifications of intended use

Anchorages subject to:

- · Static and quasi-static loads.
- Fire exposure

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C12/15 to C50/60 according to EN 206-1:2000.
- Maximum chloride concrete of 0,40% (CL 0.40) related to the cement content according to EN 206-1:2000.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of $\phi + 60$ mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature Range:

• - 40°C to +80°C (max. short term temperature +80°C and max long term temperature +50°C).

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions or subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist
 - (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- · Verifiable calculation notes and drawings are prepared taking account of the forces to be transmitted.
- Design according to EN 1992-1-1:2004+AC:2010, EN 1992-1-2:2004+AC:2008 and Annex B 2 and B 3.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

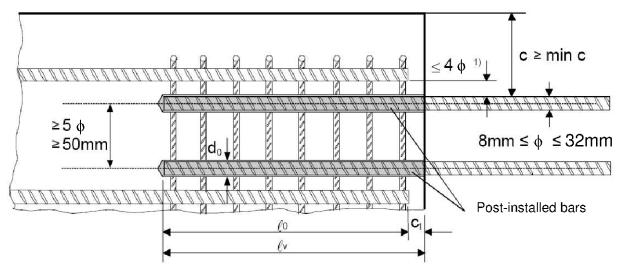
- · Dry or wet concrete.
- · It must not be installed in flooded holes.
- · Overhead installation allowed.
- · Hole drilling by hammer drill (HD) hollow drill (HDB) or compressed air drill mode (CD).
- The installation of post-installed rebar resp. tension anchors shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the Member States in which the installation is done.
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint).

Mungo Injection system MIT-Hybrid Plus for rebar connection	
Intended use Specifications	Annex B 1



Figure B1: General construction rules for post-installed rebars

- Only tension forces in the axis of the rebar may be transmitted
- The transfer of shear forces between new concrete and existing structure shall be designed additionally according to EN 1992-1-1:2004+AC:2010.
- The joints for concreting must be roughened to at least such an extent that aggregate protrude.



If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 46.

The following applies to Figure B1:

concrete cover at end-face of existing rebar C₁ minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2 min c diameter of post-installed rebar φ

lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3 ℓ_0

effective embedment depth, $\geq \ell_0 + c_1$ $\ell_{\rm v}$ nominal drill bit diameter, see Annex B 6 d_0

concrete cover of post-installed rebar

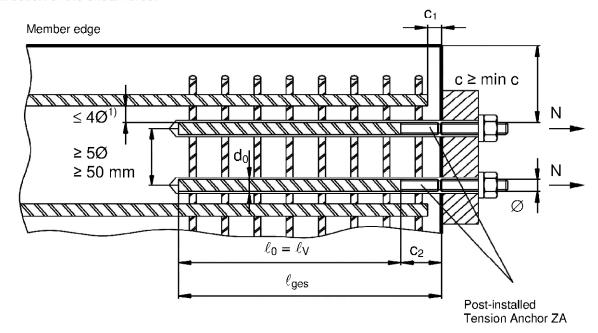
Mungo Injection system MIT-Hybrid Plus for rebar connection	
Intended use General construction rules for post-installed rebars	Annex B 2

С



Figure B2: General construction rules for tension anchors ZA

- · The length of the bonded-in thread may be not be accounted as anchorage
- Only tension forces in the direction of the bar axis may be transmitted by the tension anchor ZA
- · The tension force must be transferred via an overlap joint to the reinforcement in the building part.
- The transfer of shear forces shall be ensured by appropriate additional measures, e.g shear lugs or by anchors with an European technical assessment.
- In the anchor plate, the holes for the tension anchors shall be executed as elongated holes with axis in the direction of the shear force.



1) If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B2:

c concrete cover of tension anchor ZA

c₁ concrete cover at end-face of existing rebar

c₂ Length of bonded thread

min c minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

φ diameter of tension anchor

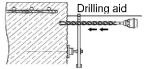
 ℓ_0 lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3

 $\begin{array}{ll} \ell_v & \text{effective embedment depth,} \geq \ell_0 + c_1 \\ \ell_{ges} & \text{overall embedment depth,} \geq \ell_0 + c_2 \\ d_0 & \text{nominal drill bit diameter, see Annex B 6} \end{array}$

Mungo Injection system MIT-Hybrid Plus for rebar connection	
Intended use General construction rules for tension anchors	Annex B 3



Table B1: Minimum concrete cover min c¹⁾ of post-installed rebar and tension anchor ZA depending of drilling method



Drilling method	Rebar diameter	Without drilling aid	With drilling aid
Hammer drilling (HD) Hollow drill bit system (HDB)	< 25 mm	30 mm + 0,06 · $\ell_{\rm v}$ ≥ 2 ϕ	30 mm + 0,02 · ℓ_{v} ≥ 2 ϕ
	≥ 25 mm	40 mm + 0,06 · $\ell_{\rm v}$ ≥ 2 ϕ	40 mm + 0,02 · ℓ_{v} ≥ 2 ϕ
Compressed air drilling (CD)	< 25 mm	50 mm + 0,08 · ℓ _v	50 mm + 0,02 · ℓ _v
Compressed all drilling (CD)	≥ 25 mm	60 mm + 0,08 · ℓ _v	60 mm + 0,02 · ℓ _v

see Annex B2, Figures B1 and Annex B3, Figure B2
Comments: The minimum concrete cover acc. EN 1992-1-1:2004+AC:2010 must be observed

Table B2: maximum embedment depth $\ell_{v,max}$

Rebar	Tension anchor	/ [mm]
ф	ф	$\ell_{v,max}$ [mm]
8 mm		1000
10 mm		1000
12 mm	ZA-M12	1000 ¹⁾ / 1200
14 mm		1000 ¹⁾ / 1400
16 mm	ZA-M16	1000 ¹⁾ / 1600
20 mm	ZA-M20	1000 ¹⁾ / 2000
22 mm		1000 ¹⁾ / 2000
24 mm		1000 ¹⁾ / 2000
25 mm	ZA-M24	1000 ¹⁾ / 2000
28 mm		1000 ¹⁾ / 2000
32 mm		1000 ¹⁾ / 2000

¹⁾ maximum embedment depth for use with hollow drill bit system (HDB)

Table B3: Base material temperature, gelling time and curing time

Concrete	temperature		temperature		ete temperature		e temperature		Gelling working time ¹⁾	Minimum curing time in dry concrete	Minimum curing time in wet concrete
- 5 °C	to	- 1 °C	50 min	5 h	10 h						
0 °C	to	+ 4 °C	25 min	3,5 h	7 h						
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h						
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h						
+ 15 °C	to	+ 19 °C	6 min	40 min	60 min						
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min						
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min						
Cartridge	temp	oerature		+5°C to +40°C							

¹⁾ t_{del}: maximum time from starting of mortar injection to completing of rebar setting.

Mungo Injection system MIT-Hybrid Plus for rebar connection	
Intended use Minimum concrete cover, maximum embedment depth, working time and curing times	Annex B 4



	Ha	nd tool	Pneumatic tool
Coaxial cartridges 150, 280, 300 up to 333 ml	e.g. Type h	1 297 or H244C	e.g. Type TS 492 X
Coaxial cartridges	e.g. Type I	257 01 112 113	e.g. Type 10 452 X
380 up to 420 ml	a a Type CCM 290/10	a g. Type H 295 or H244C	o a Type TS 495 LV
	e.g. Type CCM 380/10	e.g. Type H 285 or H244C	e.g. Type TS 485 LX
Side-by-side cartridges 235, 345 ml		- R	
	e.g. Type CBM 330A	e.g. Type H 260	e.g. Type TS 477 LX
Side-by-side cartridge 825 ml	-	-	
			e.g. Type TS 498X
All cartridges could also t	be extruded by a battery too	l.	
Cleaning and install	ation tools		_
drill bit and a class M va	em contains the Mungo MHF acuum with minimum negati	P-Clean / MHX-Clean hollow ve pressure of 230 hPa <u>and</u>	Simulation
flow rate of minimum 61			
Brush MIT-BS:	L	SDS Plus Ada	oter:
	L	SDS Plus Ada	oter:
	L ########		oter:
Brush MIT-BS:			oter:
Brush MIT-BS: Brush extension:	volume 750 ml)		air tool
Brush MIT-BS: Brush extension: Hand pump	volume 750 ml)	Rec. compressed hand slide valve (air tool



Table B5:	: Brushes, piston plugs, max anchorage depth and mixer extension, hammer
	(HD) and compressed air (CD) drilling

Bar Tension			ill Ø	d	L	d _{b,min} min.	Piston		Cartr All s	idge: izes		side	rtridge: -by-side 25 ml)
size	anchor	DIL	-Ø	Brus		Brush -	plug		or battery tool	Pneu	natic tool	Pneur	natic tool
ф	Ф	HD	CD			8		I _{v,max}	Mixer extension	I _{v,max}	Mixer extension	I _{v,max}	Mixer extension
[mm]	[mm]	[m	m]	MIT-	[mm]	[mm]	MIT-	[mm]		[mm]		[mm]	
8		12	-	BS12	13,5	12,5	-			800		800	VI 10/0 7E
10		14	-	BS14	15,5	14,5	VS14				1000 VL10/0,75	1000	VL10/0,75
12	ZA-M12	1	6	BS16	17,5	16,5	VS16	700		1000		1200	
14		1	8	BS18	20,0	18,5	VS18					1400	
16	ZA-M16	2	0	BS20	22,0	20,5	VS20					1600	
20	ZA-M20	25	-	BS25	27,0	25,5	VS25		VL10/0,75				
	ZA-IVIZU	-	26	BS26	28,0	26,5	VS25		VL10/0,73	700	VE10/0,73		VL16/1,8
22		2	8	BS28	30,0	28,5	VS28					2000	VE10/1,0
24		3	2	BS32	34,0	32,5	VS32	500					
25	ZA-M24	3	2	BS32	34,0	32,5	VS32			500			
28		3	5	BS35	37,0	35,5	VS35			300		2000	
32		4	0	BS40	43,5	40,5	VS40					2000	

Table B6: Brushes, piston plugs, max anchorage depth and mixer extension, hammer drilling with hollow drill bit system (HDB)

Bar	l hit (2		Ø d _b	d _{b,min}				ridge: sizes		Cartridge: side-by-side (825 ml)		
size	anchor	טונ - אט		Brush -	Piston plug		or battery tool	Pneu	matic tool	Pneui	matic tool	
ф	ф	HDB		Ø		I _{v,max}	Mixer extension	I _{v,max}	Mixer extension	I _{v,max}	Mixer extension	
[mm]	[mm]	[mm]			MIT-	[mm]		[mm]		[mm]		
8		12			_			800		800	VL10/0,75	
10		14	No cleaning	VS14					1000	VL10/0,75		
12	ZA-M12	16			VS16	700		1000		1000		
14		18				VS18			1000		1000	
16	ZA-M16	20					VS20					1000
20	ZA-M20	25	required	d	VS25		VL10/0,75	700	VL10/0,75			
22		28			VS28			700		1000	VL16/1,8	
24		32			VS32	500				1000		
25	ZA-M24	32		-	VS32	300		500	500			
28		35			VS35			500		1000		
32		40			VS40	1				1000		

Mungo Injection system MIT-Hybrid Plus for rebar connection

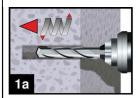
Intended Use
Parameter brushes, piston plugs, max anchorage depth and mixer extension

Annex B 6



A) Bore hole drilling

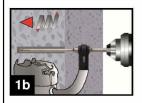
Note: Before drilling, remove carbonated concrete and clean contact areas (see Annex B1) In case of aborted drill hole: the drill hole shall be filled with mortar.



1a. Hammer (HD) or compressed air drilling (CD) Drill a hole into the base material to the size and embedment depth required by the selected reinforcing bar with carbide hammer drill (HD) or a compressed air drill (CD). Proceed with Step 2.



Hammer drill (HD + HDB)



1b. Hollow drill bit system (HDB) (see Annex B 5) Drill a hole into the base material to the size and embedment depth required by the selected reinforcing bar. This drilling system removes the dust and cleans the bore hole during drilling. Proceed with Step 3.



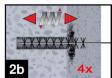
Compressed air drill (CD)

B) Bore hole cleaning

MAC: Cleaning for bore hole diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_s$



2a. Starting from the bottom or back of the bore hole, blow the hole clean a hand pump (Annex B 7) a minimum of four times.



2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B5) a minimum of four times in a twisting motion.

If the bore hole ground is not reached with the brush, a brush extension shall be used.

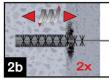


2c. Finally blow the hole clean again with a hand pump (Annex B 7) a minimum of four times.

CAC: Cleaning for all bore hole diameter and bore hole depth



2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.



2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B5) a minimum of two times. If the bore hole ground is not reached with the brush, a brush extension shall be used (Table B5).



2c. Finally blow the hole clean again with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.

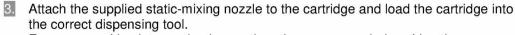
Mungo Injection system MIT-Hybrid Plus for rebar connection

Intended Use Installation instruction Annex B 7

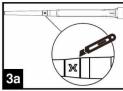


C) Preparation of bar and cartridge

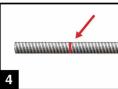




For every working interruption longer than the recommended working time (Table B3) as well as for every new cartridges, a new static-mixer shall be used.

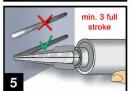


3a. In case of using the mixer extension VL16/1,8, the tip of the mixer nozzle has to be cut off at position "X".



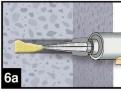
4. Prior to inserting the reinforcing bar into the filled bore hole, the position of the embedment depth shall be marked (e.g. with tape) on the reinforcing bar and insert bar in empty hole to verify hole and depth ℓ_v .

The reinforcing bar should be free of dirt, grease, oil or other foreign material.

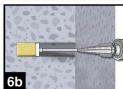


5. Prior to dispensing into the anchor hole, squeeze out separately the mortar until it shows a consistent grey colour, but a minimum of three full strokes, and discard non-uniformly mixed adhesive components.

D) Filling the bore hole

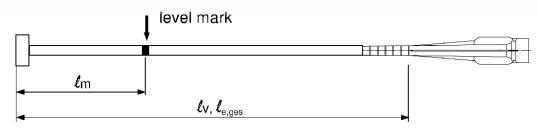


Starting from the bottom or back of the cleaned anchor hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used.



For overhead and horizontal installation and bore holes deeper than 240 mm a piston plug and the appropriate mixer extension must be used.

Observe the gel-/ working times given in Table B3.



Injection tool must be marked by mortar level mark $\ell_{\rm m}$ and anchorage depth $\ell_{\rm v}$ resp. $\ell_{\rm e,ges}$ with tape or marker.

Quick estimation: $\ell_m = 1/3 \cdot \ell_v$ Continue injection until the mortar level mark ℓ_m becomes visible.

Optimum mortar volume: $\ell_{\rm m} = \ell_{\rm v} {\rm resp.} \ \ell_{\rm e,ges} \cdot \left(1,2 \cdot \frac{\varphi^2}{d_0^2} - 0,2 \right) \ [{\rm mm}]$

Mungo Injection system MIT-Hybrid Plus for rebar connection

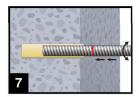
Intended Use

Installation instruction

Annex B 8

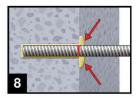


E) Inserting the rebar

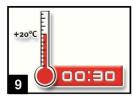


Push the reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached.

The bar should be free of dirt, grease, oil or other foreign material.



Be sure that the bar is inserted in the bore hole until the embedment mark is at the concrete surface and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed. For overhead installation fix embedded part (e.g. wedges).



Observe gelling time t_{gel} . Attend that the gelling time can vary according to the base material temperature (see Table B3). It is not allowed to move the bar after geling time t_{gel} has elapsed.

the adhesive to cure to the specified time prior to applying any load. Do not move or load the bar until it is fully cured (attend Table B3). After full curing time \mathbf{t}_{cure} has elapsed, the add-on part can be installed.

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Annex B 9



Minimum anchorage length and minimum lap length

The minimum anchorage length $\ell_{b,min}$ and the minimum lap length $\ell_{0,min}$ according to EN 1992-1-1:2004+AC:2010 ($\ell_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $\ell_{0,min}$ acc. to Eq. 8.11) shall be multiply by the amplification factor α_{lb} according to Table C1.

Table C1: Amplification factor α_{lb} related to concrete class and drilling method

Concrete class	Drilling method	Bar size	Amplification factor α _{lb}
C12/15 to C50/60	All drilling method	8 mm to 32 mm ZA-M12 to ZA-M24	1,0

Table C2: Reduction factor kb for all drilling methods

Rebar - Ø				Co	ncrete cla	ISS			
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 32 mm ZA-M12 to ZA-M24					1,0				

Table C3: Design values of the ultimate bond stress f_{bd,PIR} in N/mm² for all drilling methods and for good conditions

 $f_{bd,PIR} = k_b \cdot f_{bd}$

with

f_{bd}: Design value of the ultimate bond stress in N/mm² considering the concrete classes and the rebar diameter according to EN 1992-1-1:2004+AC:2010. (for all other bond conditions multiply the values by 0.7)

K_b: Reduction factor according to Table C2

Rebar - Ø		Concrete class							
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 32 mm ZA-M12 to ZA-M24	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3

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Performances	Annex C 1
Amplification factor α_{lb}	
Design values of ultimate bond resistance f _{bd,PIR}	



Design value of the ultimate bond stress $f_{bd,fi}$ under fire exposure for concrete classes C12/15 to C50/60, (all drilling methods):

The design value of the bond stress f_{bd,fi} under fire exposure has to be calculated by the following equation:

$$f_{bd,fi} = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \gamma_c / \gamma_{M,fi}$$

with: $\theta \le 364^{\circ}\text{C}$: $k_{fi}(\theta) = 30.34 \cdot e^{(\theta \cdot \cdot \cdot 0.011)} / (f_{bd,PIR} \cdot 4.3) \le 1.0$

 $\theta > 364^{\circ}C$: $k_{fi}(\theta) = 0$

 $f_{bd,fi}$ Design value of the ultimate bond stress in case of fire in N/mm²

 θ Temperature in °C in the mortar layer.

 $k_{fi}(\theta)$ Reduction factor under fire exposure.

 $f_{bd,PIR} \qquad \text{Design value of the ultimate bond stress in N/mm}^2 \text{ in cold condition according to Table C3}$

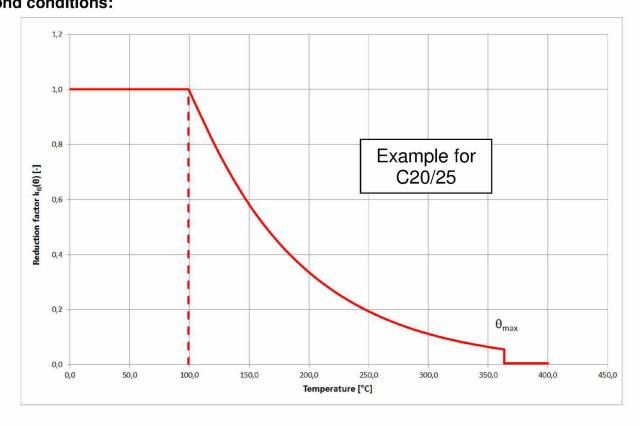
considering the concrete classes, the rebar diameter and the bond conditions according to

EN 1992-1-1:2004+AC:2010.

 γ_c partially safety factor according to EN 1992-1-1:2004+AC:2010 partially safety factor according to EN 1992-1-2:2004+AC:2008

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress f_{bd.fi}.

Example graph of Reduction factor $k_{\text{fi}}(\theta)$ for concrete classes C20/25 for good bond conditions:



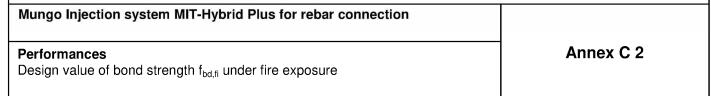




Table C4: Characteristic tension strength for tension anchor ZA under fire exposure,

concrete classes C12/15 to C50/60, according to Technical Report TR 020

Tension Anchor				M12	M16	M20	M24	
Steel, zinc plated	(ZA vz)							
	R30			20				
Characteristic	R60	_	[N1/2]	15				
steel strength	R90	$oldsymbol{\sigma}_{ extsf{Rk,s,fi}}$	[N/mm²] —		1:	3		
	R120				10			
Stainless Steel (Z	A A4 or Z	A HCR)						
	R30			30				
Characteristic	R60			25				
steel strength	R90	$\sigma_{\scriptscriptstyle{Rk},s,fl}$	σ _{Rk,s,fl} [N/mm²] 20)				
	R120				1 €	ŝ		

Design value of the steel strength $\sigma_{\mbox{\tiny Rd,s,fi}}$ under fire exposure

The design value of the steel strength $\sigma_{\text{Rd,s,fi}}$ under fire exposure has to be calculated by the following equation:

$$\sigma_{\rm Rd,s,fi} = \sigma_{\rm Rk,s,fi} \, / \, \gamma_{\text{M},fi}$$

with:

σ_{Rk,s,fi} characteristic steel strength according to Table C4

 $\gamma_{M,fi}$ partially safety factor according to EN 1992-1-2:2004+AC:2008

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Performances	Annex C 3
Design value of the steel strength $\sigma_{\text{Rd,s,fi}}$ for tension anchor ZA under fire exposure	